Active Damping of Suspension Bridges

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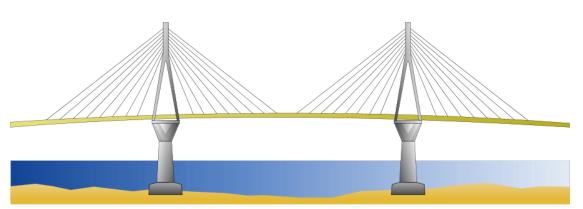


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- Conclusion



Cable-stayed bridge





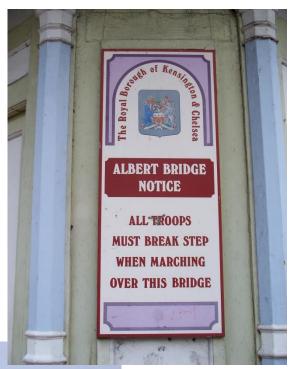
Suspension bridge





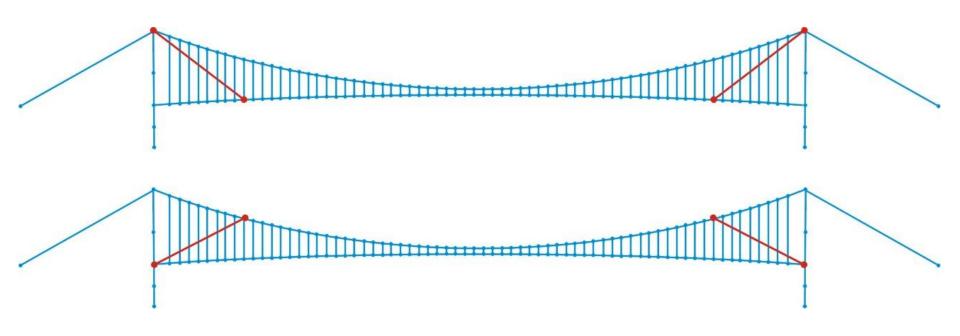
Stay cables in suspension bridges





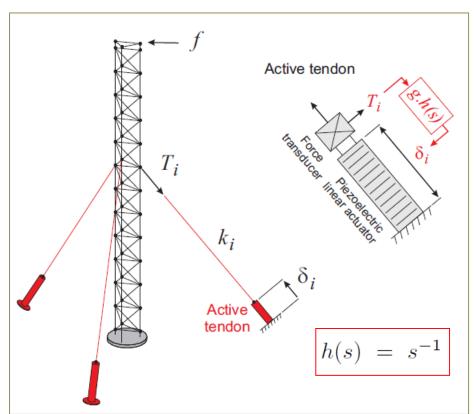


Motivation





Decentralized Collocated IFF control of cable structures



with the active cables $1 + g \frac{(s^2 + \omega_i^2)}{s(s^2 + \Omega_i^2)} = 0$ Ω_{i} Im(s) ξ_{i}^{\max} ω_{i} Re(s) active cables removed

natural frequency

Maximum achievable damping:





Recovering static stiffness: The "Beta" controller

IFF Controller:

$$h(s) = s^{-1}$$



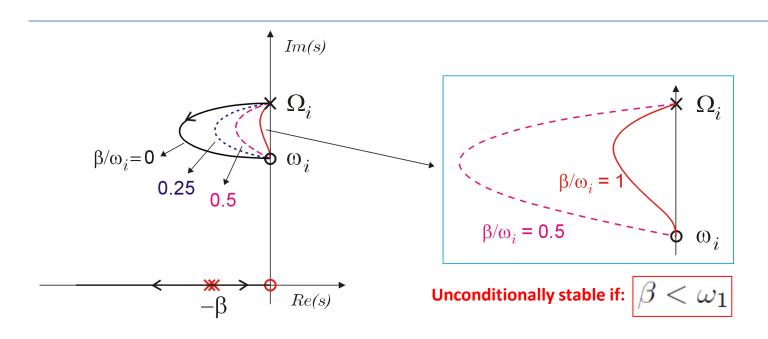
Beta Controller:
$$gh(s) = \frac{gs}{(s+\beta)^2}$$

$$\lim_{s=0} [Ms^{2} + K + \frac{s}{s+q}BK_{c}B^{T}] = K$$

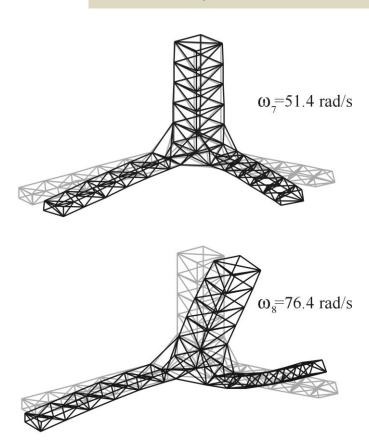
$$1 + g \frac{s^2 + \omega_i^2}{s(s^2 + \Omega_i^2)} = 0$$

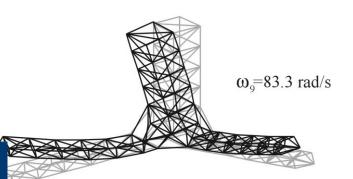
$$\lim_{s=0} \left[Ms^2 + K + \frac{(s+\beta)^2}{qs + (s+\beta)^2} BK_c B^T \right] = K + BK_c B^T$$

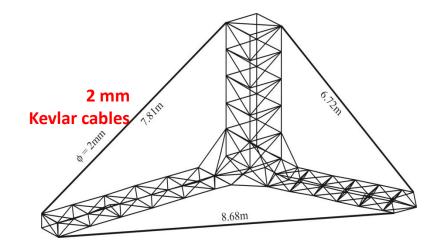
$$1 + g \frac{s(s^2 + \omega_i^2)}{(s+\beta)^2(s^2 + \Omega_i^2)} = 0$$

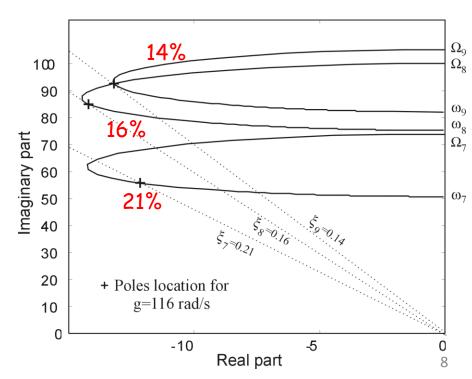


JPL Microprecision Interferometer testbed (simulation)









ULB

ULB free floating truss

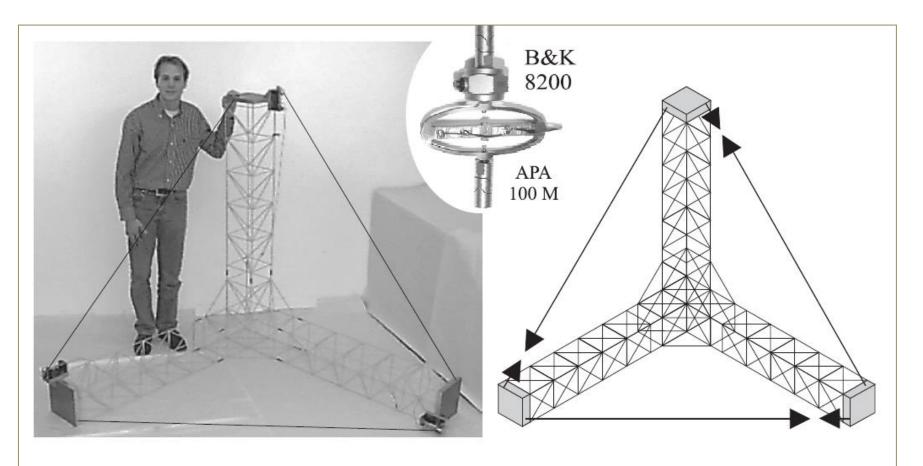


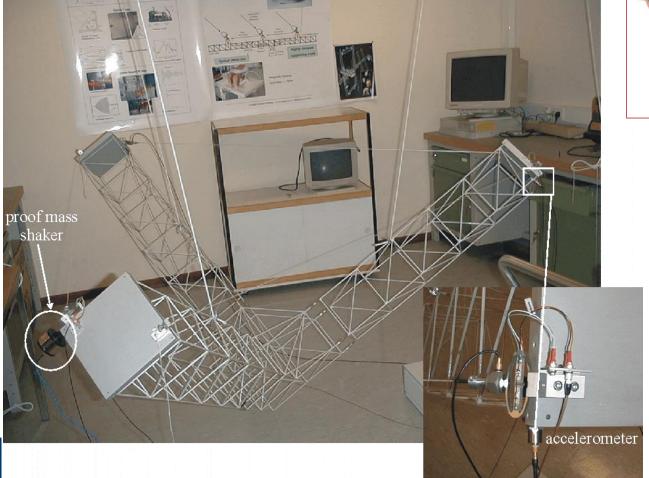
Fig. 15.13 ULB free floating truss test structure and detail of the active tendon.



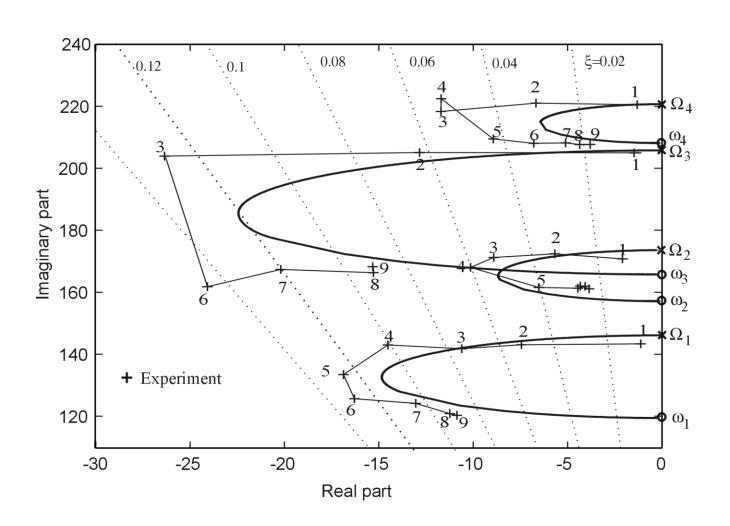
ULB free floating truss







ULB free floating truss (Theory vs. Experiment)







MAGIC – Major Atmospheric Gamma-ray Imaging Cherenkov Telescope



Focal length:17 m

Mirror diameter: 17 m

Camera Mass: 0.75 t

Total Mass:72 t

Mirror support: CFR tubes

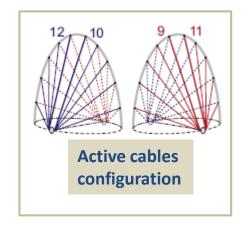
Camera Mast – Stiffened by

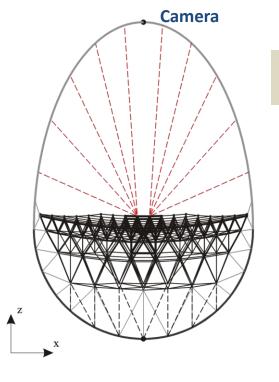
prestressed cables

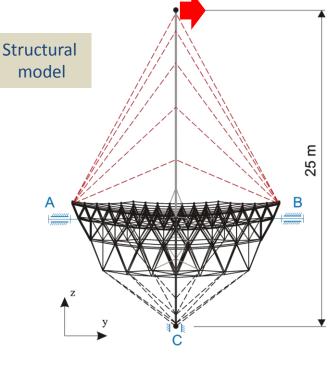
M. SMRZ, et al. Active Damping of the Camera Support Mast of a Cherenkov Gamma Ray Telescope, *Nuclear Instruments and Methods in Physics Research*, A 635 (2011)

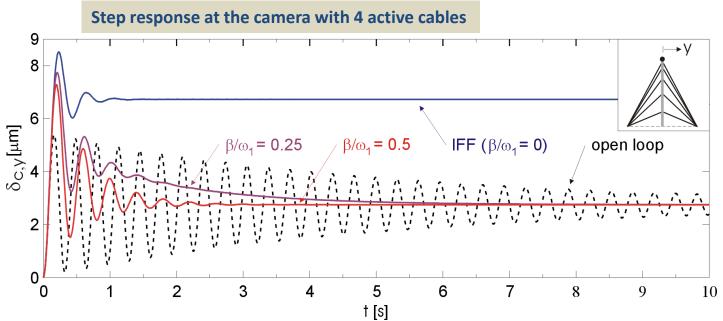


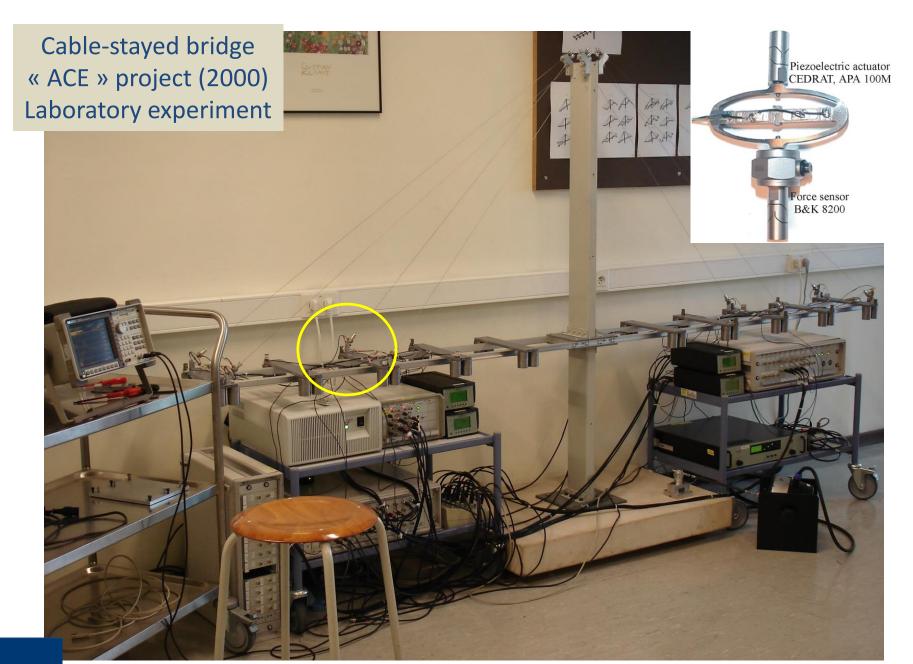
Magic Telescope

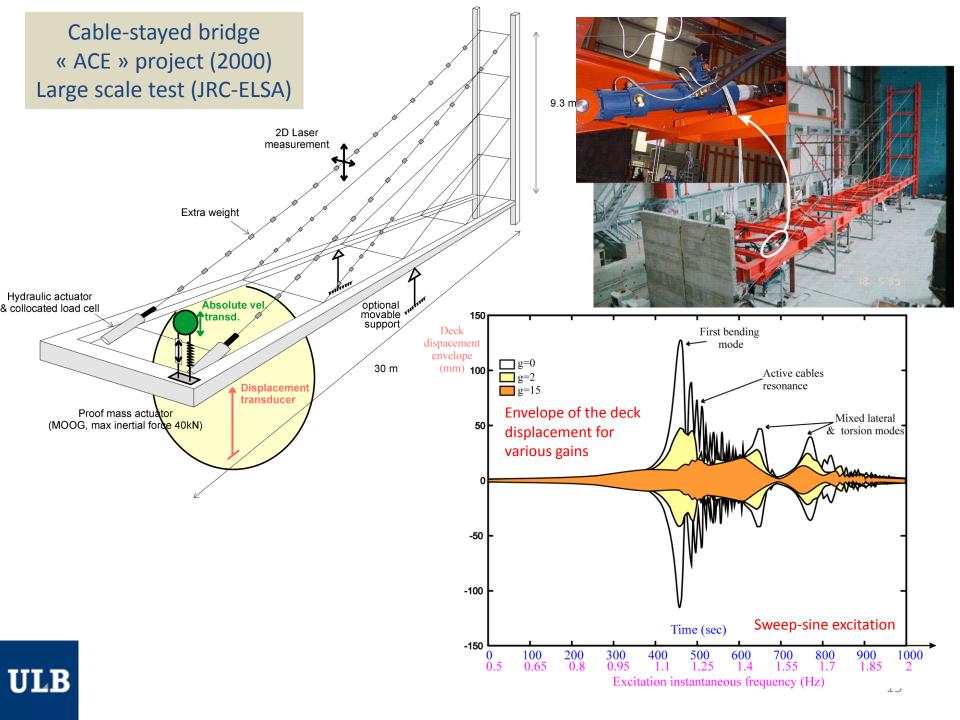




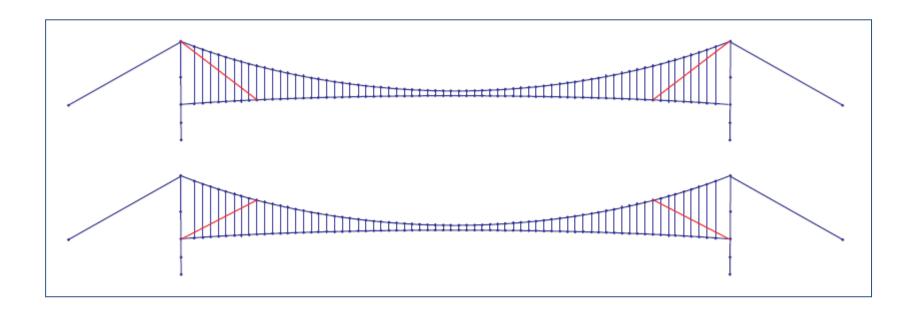








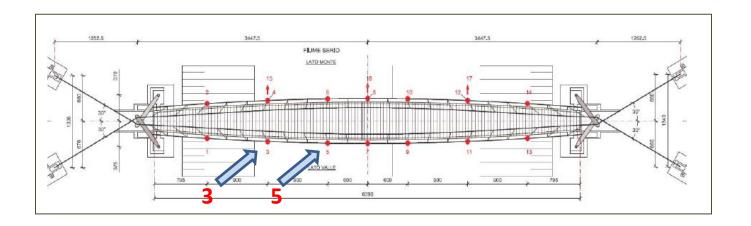
Suspension bridge

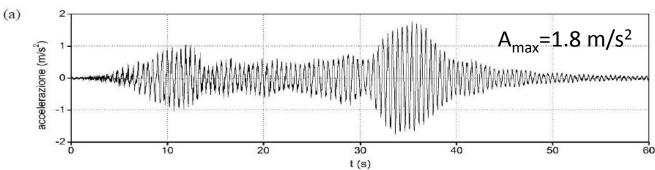




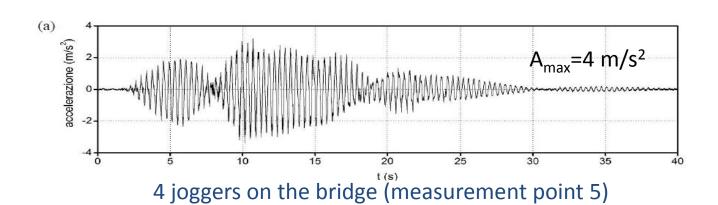
Suspension bridge: Seriate footbridge (45°40'18.5"N 9°43'45.2"E)





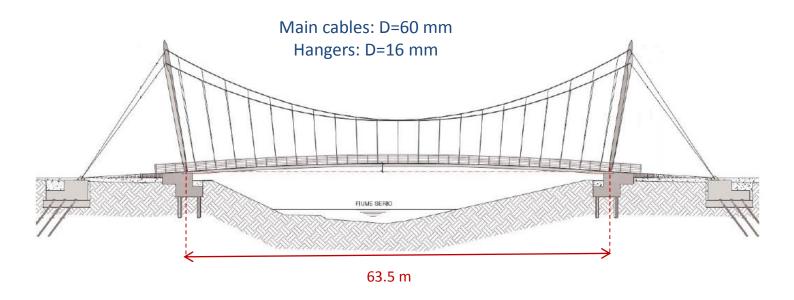


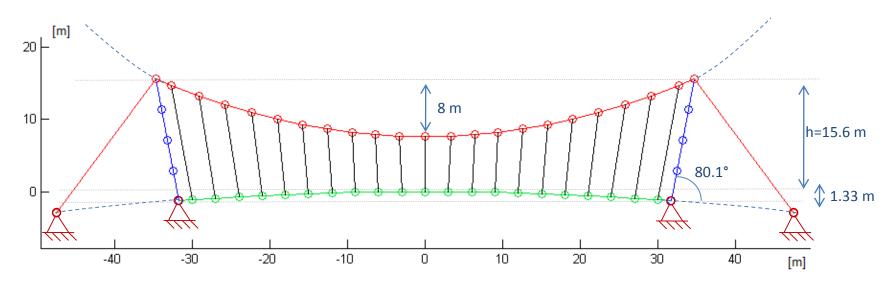
8 pedestrians walking on the bridge (measurement point 3)











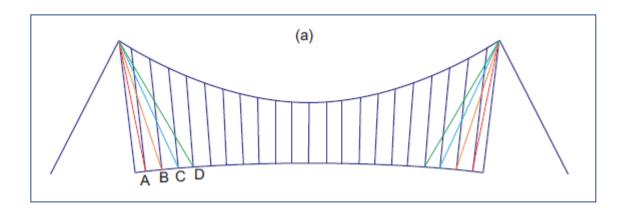
Mode	2D Numerical	3D Numerical	Experimental	Numerical	Experimental
N.	(Hz)	(Hz)	(Hz)	Mode Shape	Mode Shape
1 st B.	1.03	1.02	$\xi_1 = 2.77 \%$		
2 nd B.	1.39	1.48	1.48 $\xi_2 = 1.34 \%$		
1^{st} T.	/	1.79	1.92		
2^{nd} T.	/	2.1	1.94		
3^{rd} B.	2.22	2.20	2.17 $\xi_3 = 1.48 \%$		
3^{rd} T.	/	2.65	2.75		
4^{th} B.	2.81	2.78	2.86 $\xi_4 = 1.50 \%$		

Table 1: Natural frequencies and mode shapes of the Seriate footbridge, comparison of the 3D model and 2D model with experiments [18]. The two critical modes are **3B** and **4B**.



Option 1: Four active steel cables of diameter 10 mm between the pylon and the deck

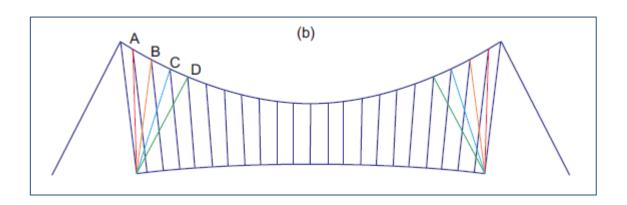
$$\xi_i^{max} = \frac{\Omega_i - \omega_i}{2\omega_i}$$



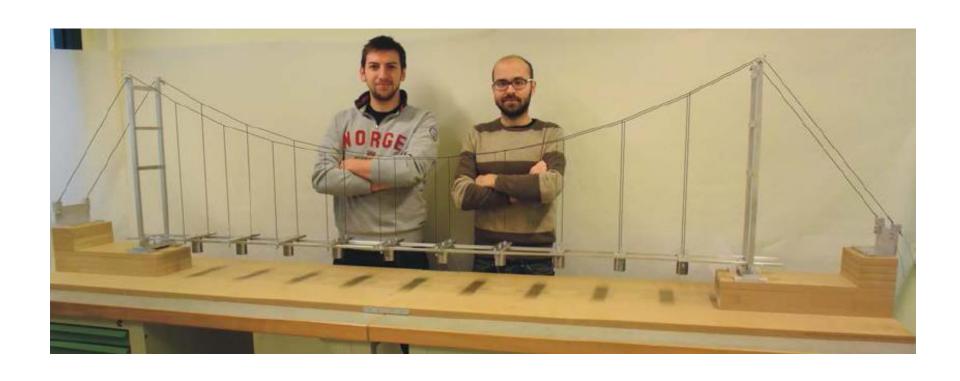
		Posit	Position A		Position B		Position C		Position D	
Mode	ω_i	Ω_i	ξ_i^{max}	Ω_i	ξ_i^{max}	Ω_i	ξ_i^{max}	Ω_i	ξ_i^{max}	
#	(Hz)	(Hz)	(%)	(Hz)	(%)	(Hz)	(%)	(Hz)	(%)	
1^{st} B	1.02	1.07	2.2	1.22	9.8	1.38	17.5	1.53	24.7	
2^{nd} B	1.48	1.49	0.6	1.54	2.1	1.55	2.5	1.53	1.8	
$1^{st} T$	1.79	1.81	0.6	1.91	3.3	2.04	6.9	2.12	9.0	
2^{nd} T	2.10	2.10	0.2	2.13	6.2	2.13	0.8	2.18	2.0	
$3^{rd} \; \mathrm{B}$	2.20	2.23	0.7	2.36	3.6	2.54	$\left(\frac{7.7}{}\right)$	2.64	10.0	
$3^{rd} T$	2.65	2.65	0.0	2.65	0.0	2.65		2.65	0.0	
$4^{th}~\mathrm{B}$	2.78	2.85	6.3	3.13	6.3	3.31	$\left(\underline{9.6}\right)$	3.17	(7.1)	
$4^{th} \mathrm{T}$	3.26	3.28	1.7	3.37	1.7	3.52	3.9	3.66	6.1	

Option 2: Active cables attached to the catenary

$$\xi_i^{max} = \frac{\Omega_i - \omega_i}{2\omega_i}$$



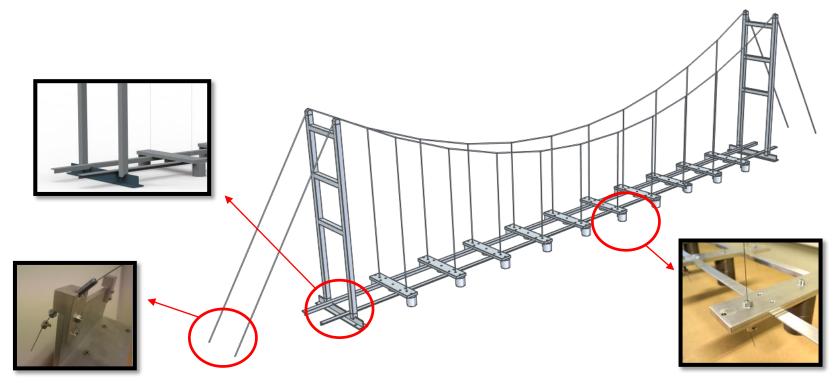
		Position A		Position B		Position C		Position D	
Mode	ω_i	Ω_i	ξ_i^{max}	Ω_i	ξ_i^{max}	Ω_i	ξ_i^{max}	Ω_i	ξ_i^{max}
#	(Hz)	(Hz)	(%)	(Hz)	(%)	(Hz)	(%)	(Hz)	(%)
1^{st} B	1.02	1.06	1.6	1.21	9.4	1.40	18.5	1.58	27.2
2^{nd} B	1.48	1.50	0.6	1.56	2.9	1.59	4	1.58	3.4
$1^{st} T$	1.79	1.81	0.5	1.93	3.7	2.12	9.1	2.36	15.7
2^{nd} T	2.10	2.11	0.3	2.16	1.5	2.18	1.9	2.16	1.5
$3^{rd} \; \mathrm{B}$	2.20	2.21	0.3	2.30	2.4	2.42	5.1	2.90	(15.9)
$3^{rd} T$	2.65	2.65	0.0	2.66	0.0	2.66	0.1	2.66	0.1
$4^{th} \; \mathrm{B}$	2.78	2.83	1.0	3.09	5.7	3.63	15.4	3.59	(14.7)
$4^{th} \mathrm{\ T}$	3.26	3.27	0.1	3.35	1.3	3.54	4.3	3.81	8.3



MSc students Andrea Sangiovanni and Matteo Voltan, from Politecnico di Milano (2015)

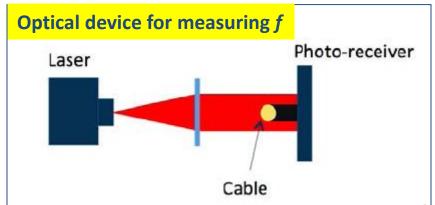


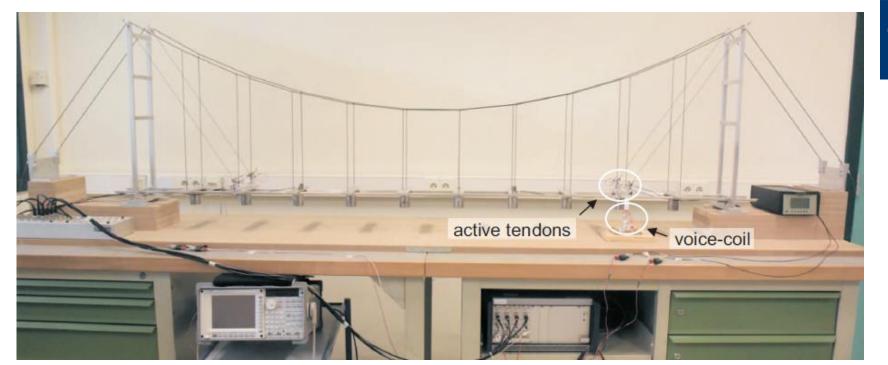
Schematic view of the laboratory mock-up



The tension in the hanger is adjusted from the measured natural frequency:

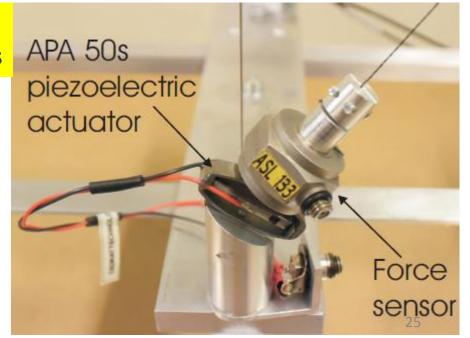
$$f = \frac{1}{2L} \sqrt{\frac{T_0}{\varrho A}}$$







Mock-up with 4 active tendons



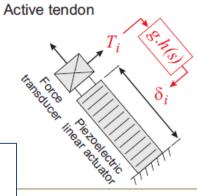
Laboratory demonstrator: comparison between numerical and experimental modes

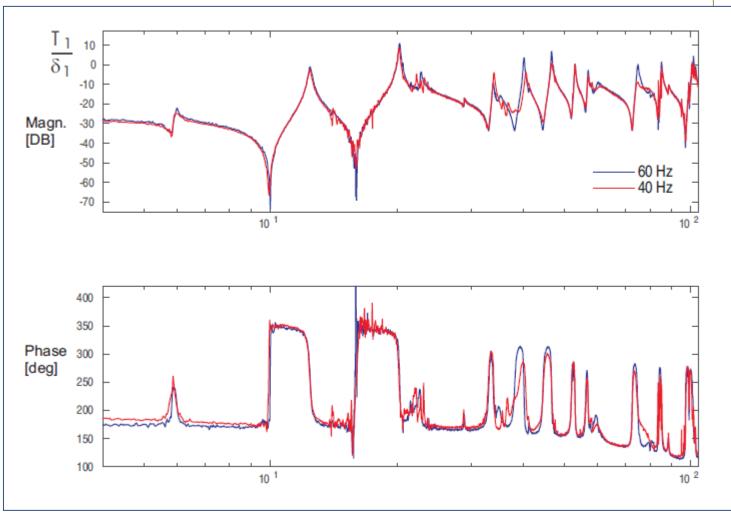
Mode N.	Numerical (Hz)	Experimental (Hz)	Numerical Mode Shape	Experimental Mode Shape
1 st B.	4.84	4.81		
2 nd B.	7.68	5.59		
3 rd B.	11.33	10.82		
4 th B.	17.93	18.25		
3^{rd} T.	19.12	21.75		
5 th B.	28.01	28.84		



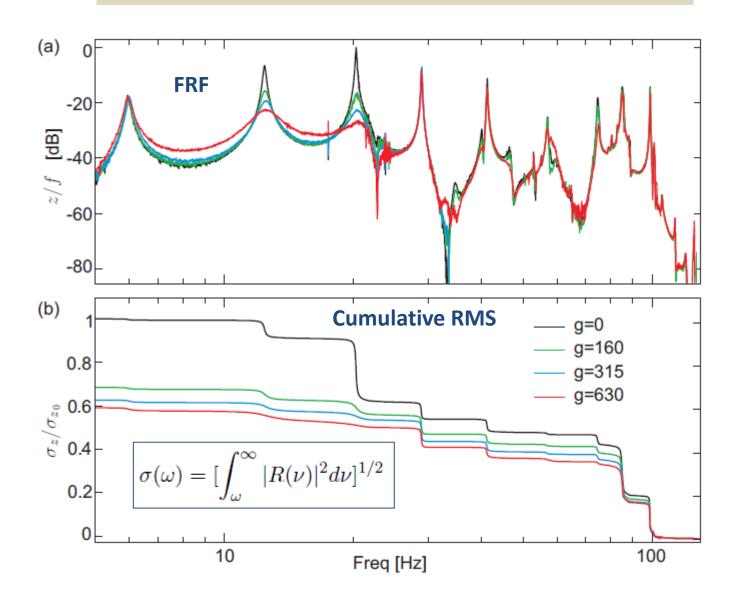
Open-loop Transfer Function



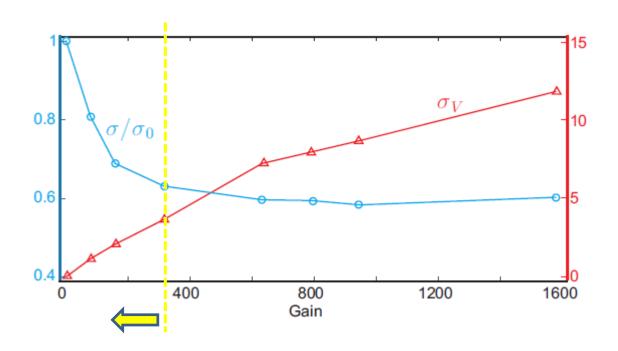




Response to disturbance, z/f with a single loop of control

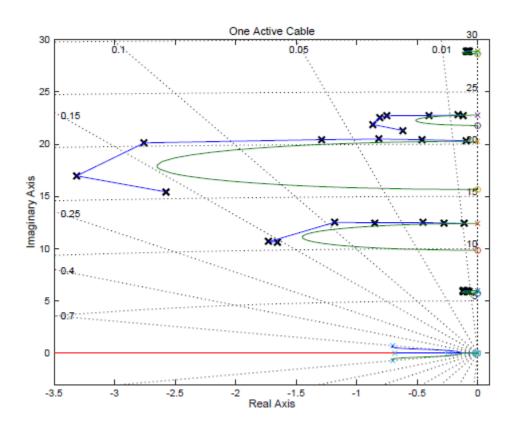


Response with a single control loop: Evolution of the RMS response (z) and the RMS control input (v) with the control gain g



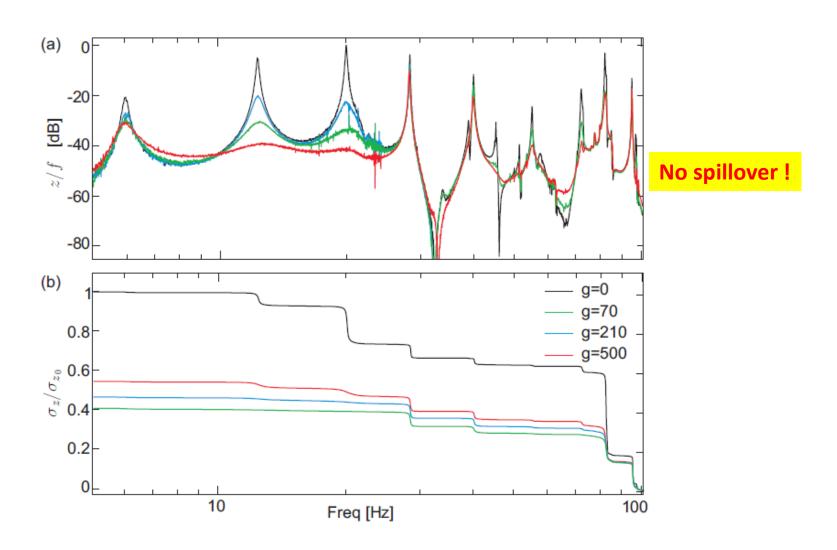


Response with a single control loop: Root locus reconstruction and comparison with the approximate theory





Decentralized control with Four independent loops Reponse to disturbance z/f (FRF and cumulative RMS)



Conclusions

- Decentralized active tendon control of cable-structures is possible.
- Simple prediction formulae based on linear models may be used for design.
- The simple performance prediction formulae are supported by experiments.
- The static stiffness deficiency of the IFF can be recovered by high –pass filtering.
- A highly effective control of a **suspension bridge** may be obtained with few and small active control cables which do not have to withstand the dead loads.



Acknowledgements

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